



**GOVERNMENT OF TELANGANA
TELANGANA DRINKING WATER SUPPLY PROJECT
Rural Water Supply & Sanitation Department**

TELANGANA WATER GRID



**L&T Construction - Water, Smart World & Communication
CHENNAI**

CLIENT:
RURAL WATER SUPPLY AND SANITATION DEPARTMENT (WATER
GRID), TELUNGANA.

CONSULTANT :
WAPCOS LIMITED

PROJECT : PROVIDING DRINKING WATER TO HABITATIONS IN KOMARAMBHEEM ASIFABAD SEGMENT IN
ADILABAD DISTRICT

SUPPLIER /
CONTRACTOR: L&T Construction, Water, Smart World and Communication

JOB Ref. No. : LE150883

TITLE :

	NAME	SIGN	DATE
DSGN			
CHKD			
APPD			

**DESIGN OF SUMP - 20KL CAPACITY
GANDIGUDA AT INDERVELLY MANDAL**

DOC./DRG. No.

L E 1 5 0 8 8 3 - C - W S - R W - D C - 1 4 1 2

SIZE
A4

REV.
A

RELEASED FOR

PRELIMINARY

INFORMATION

APPROVAL

CONSTRUCTION

Submitted sir,

Sub:RWS&S-TDWSP- Gandiguda 20KL clear water sump in Indervelly Mandal–
Komarambheem Asifabad Segment-Adilabad District-Designs -Approval-Reg.

Kindly pursue the Designs of the following 20KL Clear Water sump at Gandiguda(V) ,Indervelly (M), submitted by the Executive Engineer TDWSP Asifabad Division ,Adilabad district for approval.

1. 20 KL Clear Water Sump.

The Executive Engineer TDWSP Asifabad Division has submitted Structural Designs & Drawings of 20KL Clear Water sump based on the field conditions and as per the estimate provisions , the structural designs & drawings for the above structure is verified with RWS&S standard Type Designs and submitted for approval.

The following design parameters were considered:

- Capacity : 20kL
- Net SBC of Soil : 10.0 t/sqm
- Grade of concrete & Steel : M 30 & Fe 500
- Dia of sump Inner to Inner : 4.00m
- Sidewall Height : 2.2mts
- Sidewall Thickness:150mm
- Top Slab thickness: 150 mm
- Raft Slab thickness: 250mm

As per the above parameters the structural design and drawings of the clear water sump is verified, as per similar Type designs available and approved by the RWS&S Department considering the SBC and type of soil , duly following IS codes, IS: 456-2000, SP:16, 34, IS:3370 and IS 1893-2002 (seismic codes).The sizes and steel proposed in the designs and drawings of all components are safe and sufficient.

The additional points noted after checking the designs are:

- Detailed Estimate of the Structure with these specifications has to be prepared and compared with the provision made in sanctioned estimate. Such that deviation if any is within authorized limits. If any deviations noticed, the Estimate should be submitted for obtaining approval from the Competent Authority.

Subject to approval a draft memo addressed to the EE, TDWSP Asifabad Division , for communicating approved Structure is put up for kind perusal and approval.

AEE (Designs)
TDWSP, Nirmal Circle

DEE (Designs)
TDWSP, Nirmal Circle

Superintending Engineer,
TDWSP, Nirmal Circle

Design Of 20 KL Capacity Sump at (WITH FLAT TOP SLAB)

Data Not more than 5m Span

Location					
Safe bearing Capacity	sbc	safe for sbc		100 Kn/m ²	
Capacity	v			20 KL	
Free Board	fb			0.30 m	
Dead Storage	ds			0.30 m	
Dia of sump	d			4.00 m	
Projection of Bottom slab from side wall		ps = d/16 to d/8		0.25 m	OK
Depth of tank above GL	dgl	No uplift check required		1.00 m	
Depth of Water table below GL	wl	Safe Against Uplift		0.90 m	
Depth of the tank	h			2.20 m	1.20
Thickness of Slab	td	Min 150mm thick	OK	0.150 m	0.097 m
Th. Of Side wall	sth	Min 150mm thick	OK	0.150 m	0.069 m
Th. Of Bottom Slab	bsth	Min 150mm thick	thick is Sufficient OK	0.25 m	0.000 m

Top Slab

Radius of Top slab		rs		2.00	slab projection	0.15 m
Thickness of Slab		td	Min 150mm thick	OK	0.15 m	0.097 mm is required
Effective depth of slab		d		100 mm		
Total Load on slab						
			Self wt	3.75 kN/sqm		
			Live load	1.5 kN/sqm		
			Finishings	1 kN/sqm		
			Total load	<u>6.25 kN/sqm</u>		
Bending Moment		bm	(2/16*w*rs^2)	3.13 kN-m/m		
Area of Steel required		ast	(bm/(σ_{st} d))	279 sqmm/m		
			σ_{st}	N/mm ²	130.00 N/sqmm	
			j=1-k/3	0.86		
			k=1/1+ $\sigma_{st}/m\sigma_{cbc}$	0.42		
			m=280/3 σ_{cbc}	9.33		
			σ_{cbc}	10 N/sqmm		
			Grade of steel	Fe 415, σ_{st} =	130 N/sqmm	
			Grade of Concrete	M 30		
Dia of Reinforcement		db	10mm			
Spacing required		min of	150 mm	280 mm	floor to	10mm
Spacing provided				OK	150 mm	c/c
Provide 10 mm dia Tor @ 150 mm C/c both radially and in the form of circular rings						

Side Wall

Depth of the tank		h		2.2 m	
Th. Of Side wall		sth	Min 150mm thick	0.15 m	0.069 mm
Depth of tank above GL		dgl		1.00 m	

Moments

Inner Side			1.55 Kn-m
Outer Side			1.1 Kn-m

Hoop force

Inner Side			30.72 Kn	(Tension)
Outer Side			18.11 Kn	(Compression)

Reinforcement

				Dia	Spacing Provided	floor to
Inner face	Vertical	(Min 10mm dia)	240 mm ²	10 mm	150 mm	50
	Horizontal		180 mm ²	10 mm	150 mm	50
Outer face	Vertical	(Min 10mm dia)	240 mm ²	10 mm	150 mm	50
	Horizontal		180 mm ²	10 mm	150 mm	

Bottom slab

		Min 150mm thick				
Safe bearing Capacity		sbc		100 Kn/m ²		
Th. Of Bottom Slab		bsth	thick is Sufficient	0.250 m	0.000 m	
Dia of Bottom Slab		db		4.80 m		
Size of Haunch		bh		0.25 m		
effective cover to reinforcement for raft slab			covraft	65 mm		
Moments						
Radial				2.49 Kn-m		
Circumferential				2.39 Kn-m		
			Ast (Min 10mm dia)	Dia	Spacing Provided	floor to
Reinforcement Top	mesh		300 mm ²	10 mm	225 mm	25
Reinforcement Bottom	mesh		300 mm ²	10 mm	200 mm	50
						200

Provide 10 mm dia TOR @ 150 mm C/c on both faces in staggered fashion

Design Of Bottom Slab

CASE-1

Projection from side wall	ps		0.25 m
Dia of Bottom Slab	db	$d+2*sth+2*ps$	4.80 m
Size of Haunch	bh		0.25 m
Dia of Bar	top dbbs		10 mm
	bottom dbbsb		10 mm

Net Load on Bottom Slab

Wt of Top Slab		$\pi/4*(d+sth+topproj)^2*td*25$	62.32 Kn
Wt Of Side wall		$\pi*(d+sth)*sth*(h-dtrb)*25$	97.78 Kn
Wt of Haunch		$\pi*(d-bh)*bh^2/2*25$	9.21 Kn
Total Load	wbs		<u>169.31 Kn</u>
LL on TOP slab		$\pi/4*(d+sth+topproj)^2*LL$	36.30 Kn
Effective foundation width of sidewall load on bottom slab, ewf		$ps+sth+bh+bsth=$	2.056 sq m 0.158
Max Pr on Soil	prb	$Wbs/(\pi*(d+sth)*ewf)$	14.43 Kn/m ²

Bottom Slab is designed as circular Slab loaded with UDL and Simply Supported on edges

Radial moment	mri	$3/16*prb*((db/2)^2-((d+sth)/2)^2)-wbs/(8*\pi)*(2*l$	2.075	1.95
Circumferential Moment	mti	$1/16*prb*(3*(db/2)^2-((d+sth)/2)^2)-wbs/(8*\pi)*(2$	-2.49 mrb	-2.38 Kn-m
			-2.39 mtb	-2.38 Kn-m
Max Radial Moment	mr	$IF(wl>hbgl,0,CEILING(3*prb*(db/2)^2/16,0.01))$	2.49 Kn-m	
Max Circumferential moment	mt	$IF(wl>hbgl,0,CEILING(prb*(db/2)^2/16,0.01))$	2.39 Kn-m	

Base Slab Th for Uncracked Condition

Th	bsthr	$IF(mr=0,sth*1000,(max(mr,mt)*6*10^6/(2*10^3))^$	0.087 m
			Provided th is Sufficient
Eff Depth	de	$bsth*1000-covraft$	185 mm
Area of Steel			
Min Steel	Astmin	0.24	0.24 %
			300 mm ²
Area of Steel	Astr	$mr*10^6/(130*.87*de)$	121 mm ²
Spacing			
Top Steel	Asttp	$\pi*(dbbs^2/4)*1000/max(Astmn,astr)$	262 mm
Provide 10 mm dia TOR @ 225 mm c/c in the form of mesh at top			
Bottom Steel	Astb	$\pi*(dbbsb^2/4)*1000/(Astmin)$	262 mm
Provide 10 mm dia TOR @ 200 mm c/c in the form of mesh at bottom			

Check For SBC

Load from tank Portion	wbs		169.31 Kn
Weight of Bottom Slab	wbsi	$\pi*(db^2/4)*bsth*25$	113.1 Kn
Weight of water	ww	$\pi*(d^2/4)*h*10$	276.46 Kn
Total	W	$wbs+wbsi+ww$	558.87 Kn
Pr on Soil	pr s	$w/(\pi*db^2/4)$	30.89 Kn/m ²
		safe for sbc	

CASE-2

Bottom Slab is designed as circular Slab loaded with Uplift and continuous Supported on edges

Load on Bottom Slab (Uplift)

Max Pr on Soil	pru	$10*(h-dgl-wl)-bsth*25$	No uplift check rec	0.00 Kn/m ²
Radial moment	mru	$2/16*pru*(r)^2$		0
Circumferential Moment	mtu	$1/16*pru*(r)^2$		0

Base Slab Th for Uncracked Condition

Th	bsthr	$IF(mru=0,sth*1000,(max(mr,mt)*6*10^6/(2*10^3))$	0 m
			thick is Sufficient
Eff Depth	de	$bsth*1000-covraft$	185 mm
Area of Steel			
Min Steel	Astmin	0.24	0.24 %

APPROVED
M 30/14/26
SE, NIRMAL

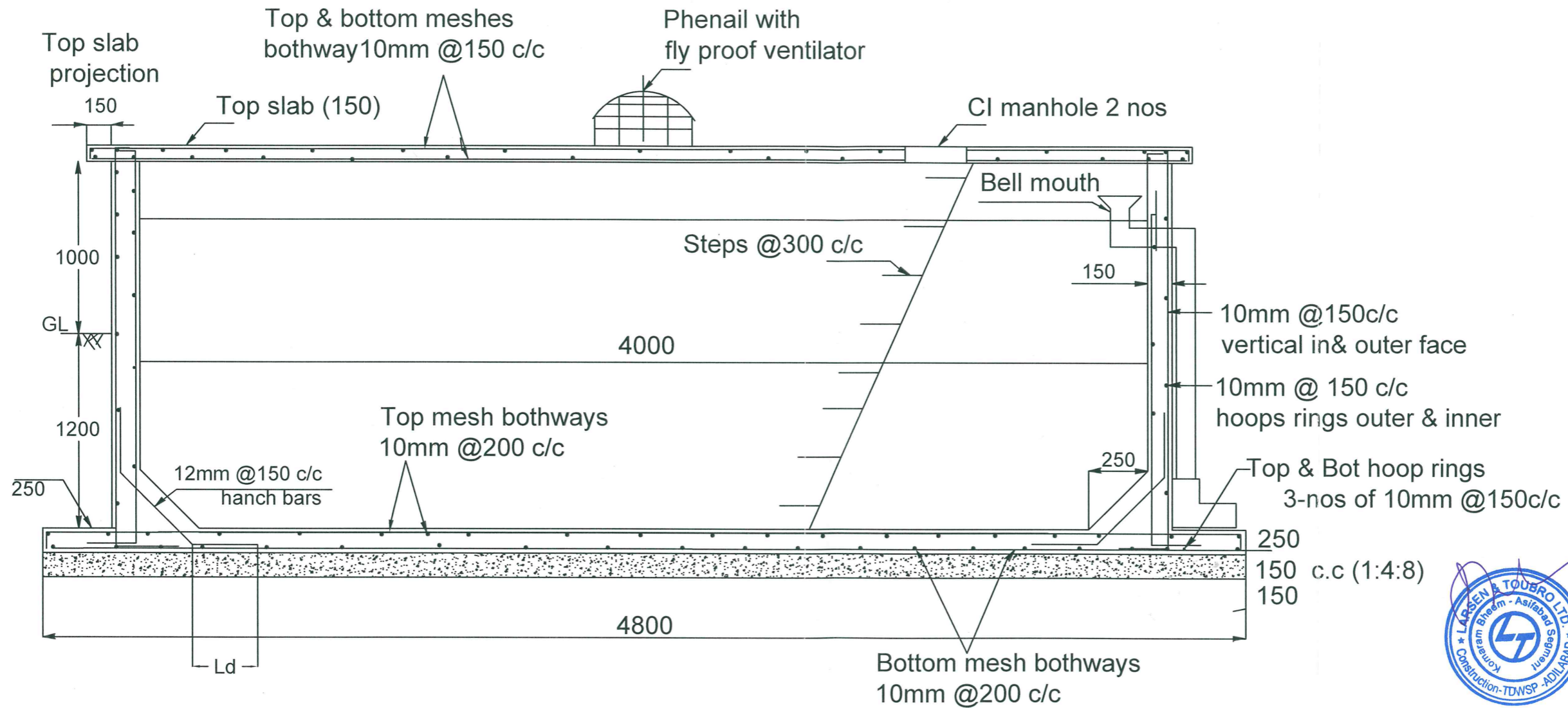


[Signature]
Asst. Executive Engineer
 TDWSP Asifabad

[Signature]
Dy. Executive Engineer
 TDWSP Asifabad

[Signature]
Executive Engineer
 TDWSP Asifabad

20KL SUMP



All dimenstions are in 'mm'
 Concrete mix V.R.C.C M30
 Steel Fe-415
 Reinforcement details shall be
 as per IS-SP34

Asst. Executive Engineer
 Asst. Executive Engineer
 TDWSP Asifabad

Dy. Executive Engineer
 Dy. Executive Engineer
 TDWSP Asifabad

Executive Engineer
 Executive Engineer
 TDWSP Asifabad

APPROVED
 20/11/11
 SE, NIRMAL



FOR INFORMATION				
REV.NO.	DESCRIPTION	DESIGNED	DRAWN	CHECKED APPROVED
REVISIONS				
L&T Construction Water, Smart World & Communication.				
CLIENT: RURAL WATER SUPPLY AND SANITATION DEPARTMENT, TELANGANA.		CONSULTANT:		
PROJECT: PROVIDING DRINKING WATER TO HABITATIONS IN KOMARAMBHEEM ASIFABAD SEGMENT IN ADILABAD DISTRICT (PRIMARY GRID)				
SUPPLIER/CONTRACTOR: L&T Construction Water & Effluent Treatment SBG				
JOB No:	LE150883	TITLE:		SCALE:
NAME	SIGN	DATE	GANDIGUDA AT INDERVELLY MANDAL SUMP - 20KL	PROJECTION
DRAWING No. LE150883-C-W-S-RW-DC-11412		SIZE	REV.	
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CHECKED BY	SIGN	DATE
CIVIL & STRUCTURAL		
MECHANICAL		
ELECTRICAL		
INSTRUMENTATION		

TELANGANA DRINKING WATER SUPPLY PROJECT
20 KL SUMP AT GANDIGUDA, INDERVELLY (M) IN ADILABAD DIST

1. INTRODUCTION

M/s. L & T Construction, Water & Effluent Treatment is proposing to construct 20 KL Sump at Wadoni (V), Utnoor (M). The work is taken up under Segment 22 , Komaram Bheem Project , TDWSP, Asifabad in Adilabad Dt.

The present Report presents the results of (3) Bore hole.


M/S Anji Drilling & Grouting works; Anantapur has carried out the drilling of bore holes, collection of soil and rock samples and conduct of Standard Penetration Tests at different levels in the respective bore holes at the proposed site.

Analysis of borehole data , Laboratory tests and geotechnical investigation report have been made by Prof. D Babu Rao, ME (IIT,R) , Ph.D. (USA), MIGS, Empanelled Consulting Geo technical Engineer & Director, Geo technologies, Former Professor of Civil Engineering, Osmania University.

2. SCOPE OF WORK

The following is the scope of work of M/s. Anji Drilling and Grouting Works:

- Drilling Borehole at (1) location for 20 KL Sump in Wadoni , Adilabad Dt.
- Conducting SPT at regular intervals, where feasible
- Collection of undisturbed / disturbed samples from the Bore holes
- Preparation of Technical Report recommending suitable foundations and safe bearing capacity


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M.E., Ph.D.(USA)
Consulting Geotechnical Engineer



Following is the scope of work of Prof. D Babu Rao ,

Testing of soil samples in the Laboratory

Preparation of Technical Report

3. SUB SOIL INVESTIGATION

The sub soil investigation was carried out to determine:

Nature of sub stratum and engineering properties of sub strata which may affect the mode of construction of the proposed work.

FIELD INVESTIGATION PROCEDURE:

The following technique is adopted for sub soil investigations.

a) BORINGS:

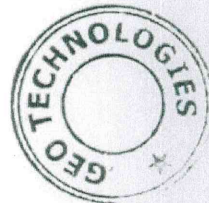
Rotary Drilling was done using TC / Diamond bits. The size of the casing used was 125 to 75 mm, yielding samples of NX size.

TC bits were employed for the overburden, and Impregnated Diamond Core bits were used for rock formation.

Drilling was performed on 7-8 Jan ,2016.

The following relevant data was recorded during Rotary drilling operations.

- Nature of strata
- Details of samples
- Core Recovery (CR)
- Rock Quality Designation (RQD)



D Babu Rao

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b) STANDARD PENETRATION TEST (SPT):

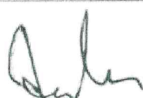
SPT split spoon sampler of standard dimensions was driven into the soil from the borehole bottom using 63.5 kg hammer with a fall of 75 cm height. The SPT weight was lifted to the specified height and allowed to fall freely on the anvil with the use of cat-head winch with one to one and half turn of the drum. Blow counts for the penetration of every 15 cm were recorded and the 'N' value is reported as the blow counts for 30 cm penetration of the sampler excluding the first 15 cm penetration as seating drive.

When the number of blows exceeded 50 to penetrate the first or second 15 cm length of the sampler, the SPT 'N' is regarded as more than 100 as described in IS 2131 - 1981. The test is terminated in such case and a record of the penetration of the sampler under 50 blows is made. SPT refusal is recorded when there is no penetration of the sampler at any stage and also when a rebound of the sounding system is recorded. These tests were conducted at close intervals of 1.0m so that a continuous SPT 'N' profile is available.

Disturbed soil collected in the SPT sampler was preserved in polythene covers and transported to the laboratory. Additional polythene cover was used to prevent the loss of moisture during the transit period.

c) DEPTH OF BORING: The depth of the Bore hole was as follows:

BH No	R L	Drilled depth
1	417.780	6 m



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d) LOG OF BORE HOLE:

All the results obtained from the field operations are presented in Log of Bore hole in Fig. 1 .

4. LABORATORY TESTING:

The laboratory tests are conducted in the laboratory of Geotechnologies, Hyderabad, an ISO- 9000 approved Laboratory.

Weathered rock from 0 – 2 m depth was followed by Hard rock to 6 m depth.

The following tests were conducted on cores from hard rock :

- Unconfined compressive strength (as per IS: 9143)

Table 1 gives the rock properties of Cores.

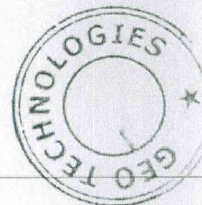
5. SUB SOIL PROFILE

Based on Field and Laboratory tests, the following idealized sub soil profile is evolved.

Depth	Strata	N value
0 – 2 m	Weathered rock	>100-
2 - 6 m	Hard rock	Cores

In Hard rock, no SPT could be conducted. However, in weathered strata, SPT was conducted with N values tending to be 'refusal'. This is the criterion for distinguishing between SDR/Weathered rock and Hard rock.


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6.0 SHALLOW FOUNDATIONS

In general, the following pertains to foundations resting in soils.

. A properly designed foundation has to satisfy the following two limit states.

- 1) Limit state of collapse (i.e. Shear strength)
- 2) Limit state of serviceability (i.e. Settlement)

SHEAR CRITERIA:

The first criterion is depends on shear strength. The calculations are based on "TERZAGHI" bearing capacity equation as recommended by IS: 6403 (with factor of Safety) which takes care of L/B ratio (shape), foundation depth etc., along with other parameters.

SETTLEMENT CRITERIA:

The intensity of loading that will cause a permissible settlement or specified settlement of the structure is termed as allowable bearing pressure. The settlement in this type of layer will be elastic settlement.


These foundation settlements are evaluated using elastic theory. The pressure distribution below the footing is assumed as 2 V: 1 H for estimating the settlement. Since rock formation is available at shallow depth. The settlement will be within the permissible limit. Hence open foundation is suitable.

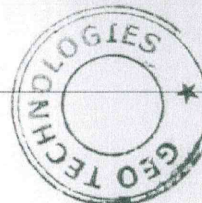
ALLOWABLE BEARING CAPACITY:

Allowable Bearing capacity (ABC) is the net intensity of the loading which the foundation will carry without undergoing settlement in excess of the permissible value for the structure under consideration but not exceeding the net safe bearing capacity (SBC).

7.0 DISCUSSION ON FOUNDATION OPTIONS

From sub soil profile and laboratory test data, it can be seen that hard rock exists


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from 2 m .Hence shallow foundation is feasible and same is recommended.

8.0 RECOMMENDATIONS

Based on Field Investigations and laboratory testing, the following Recommendations are made for construction of 20 KL Sump at Gandiguda Adilabad Dt..

a) Open foundations resting in hard rock at 2 m below GL ,are recommended. The structure is likely to result in saturation and inundation of the sub soil during long – time operation,

b) SBC is recommended as follows :

Location		BH 4,5,6
S. No.	Depth (m)	Recommended SBC t/ sq m
1	2.0	10
2	3.0	12
3	4.0	13

c) The actual size of foundations will be based on loads from the superstructure.

For ANJI DRILLING AND GROUTING WORKS



(DR. D. BABU RAO)

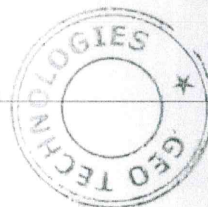
M. E (IIT,R), Ph. D. (USA), MIGS

Former Professor of Civil Engineering

Consulting Geotechnical Engineer

MCH Panelist No. 2490/TP/2000-2

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TELANGANA DRINKING WATER SUPPLY PROJECT

FIG 1 : Record of Boring, Bore Hole No : 1

20 KL SUMP AT GANDIGUDA IN ADILABAD DT.



Type of Boring: Core drilling

Dia of Boring: NX

Date : 7 – 8 Jan 2016

GL : 417.780

Drilled depth = 6 m

Depth, m	Profile	Soil	Sample Depth m	N value	CR, %	RQD%	
0		Weathered	0	>100	Small pieces		
1.0		rock	1.5				
2.0		Hard rock	2.0		79	73	
3.0			3.5		77	77	
4.0							
5.0			5.0		79	70	
6.0							
7.0							
8.0							
9.0							
10.0							
11.0							
12.0							
13.0							
14.0							
15.0							
16.0							



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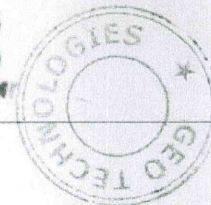
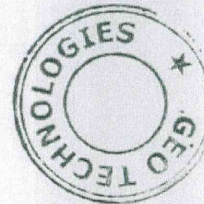


TABLE 1 : RESULTS OF TESTS ON ROCK SAMPLES
20 KL SUMP AT GANDIGUDA IN ADILABAD DT.

BH No.	Depth, m	Specific gravity	Porosity %	Water absorption %	UCS Kg / sq cm
1	2.2	2.70	3.9	2.9	410
	3.6	2.70	4.1	3.0	412
	5.4	2.71	4.0	2.9	467

NOTES : Where core Samples are less than 100 mm long, UCC tests are not conducted.


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APPENDIX

CALCULATION OF SBC

20 KL SUMP AT GANDIGUDA IN ADILABAD DT.

TYPICAL CALCULATIONS FOR OPEN FOUNDATIONS RESTING IN

HARD ROCK AT 2 M DEPTH

From Lab tests UCC is taken as 400 kg / sq cm

Keeping the heterogeneous nature of rock,

For RQD < 0.50, Field strength = 0.2 x 400 = 80 kg / sq cm

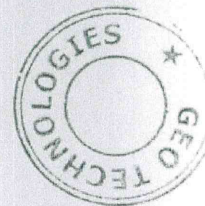
With a F.S. of 10, SBC = 80/10 = 8 kg /sq cm

= 80 t / sq m

Recommended SBC is **10 t / sq m.**



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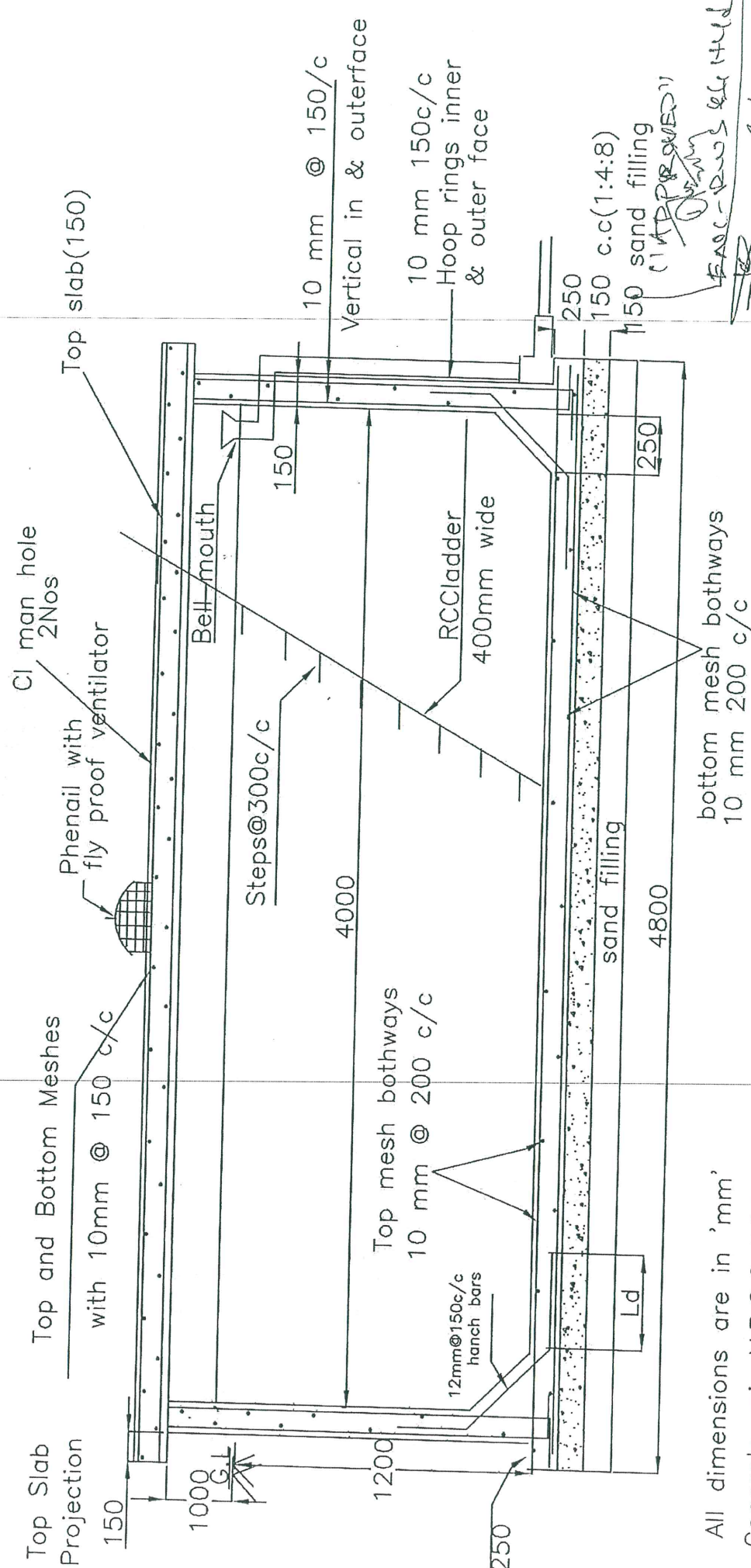
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Asst. Executive Engineer
TDWSP Asifabad

Dy. Executive Engineer
Dy. Executive Engineer
TDWSP Asifabad

Naveed
Executive Engineer
TDWSP Asifabad

APPROVED

SE, NIRMAL



All dimensions are in 'mm'
 Concrete mix V.R.C.C M30
 Steel Fe-415
 Reinforcement Details shall be as per IS - SP34

20 KL SUMP

SCHEME : CPWS Schemes 19/11/14
LOCATION: Adilabad District
DWG.NO.

10 mm @ 150/c
 Vertical in & outface
 10 mm 150c/c
 Hoop rings inner & outer face
 250
 150 c.c(1:4:8)
 150 sand filling
 (1:4:8 PROVED)
 E.A.S.C. - R.W.S. & G.H.U.D.
 19/11/14